



Stormwater Management Report

for

Fleet Street Salamander Bay

for Wanda Beach Estate Pty Ltd

Revision Schedule

Date	Revision	Issue	Prepared By	Approved By
16/09/24	A	For Approval	B. Ward	K. Sinclair
07/04/25	B	For Approval	T. van Koeverden	K. Sinclair

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1. Introduction

Northrop Consulting Engineers Pty Ltd have been engaged to undertake engineering design for the proposed development at 4 Fleet Street, Salamander Bay – contained within DP 285191. This report has been prepared to accompany a Development Application.

The purpose of this report is to summarise the proposed civil engineering solutions for submission of the Development Application to Council. The proposed management plan has been developed in accordance with Port Stephens Council's (PSC's) 2014 Development Control Plan (DCP), PSC Soil Infiltration Technical Information guidelines and relevant Australian Standards.

This management plan should be read in conjunction with the attached drawings prepared by Northrop Consulting Engineers, DA-C01.01 – DA-C09.01. We note the information contained in this report is not intended to present detailed design solutions but rather provide solutions commensurate with a design suitable for Development Application assessment.

2. Site Description

The subject site is bounded within a neighbourhood scheme off Fleet Street. The scheme bounded to the North and West by undeveloped land, to the East by existing residential developments, and to the South by Fleet Street. Figure 1 shows the development extent as well as the locality of the site.



Figure 1 – Aerial Image (NEARMAP)

The site contains existing residential developments and related ancillary structures including bin stores, car parking, internal roads and a tennis court. A ridge running from west to east splits the site in two catchments. The Southern side of the ridge generally falls to the South and Southeast, while the Northern side of the ridge generally falls North. A pit and pipe network contained within Fleet Street collects stormwater runoff from the existing development South of the ridge, while runoff heading North discharges over the boundary into Stoney Ridge Reserve.

3. Proposed Development

The proposed development comprises of 30 new multistorey residential units including incorporated garages, driveways, new road access and related landscaping. The development also includes the renovations of 11 existing cabins, extensions of pavement extents along Anchor Cove, new garages beneath 4 of the renovated cabins and 5 standalone garage units and related driveway access. The layout of the proposed development has been illustrated in the Civil Engineering Plans appended to the rear of this report.

4. Stormwater Management

General Stormwater Management Strategy

The proposed development will incorporate multiple devices and measures aimed at providing adequate and responsible management of stormwater runoff. The measures as outlined in this report are the minimum to satisfy Council requirements, however enhanced measures may also be considered in subsequent design stages for the development.

In line with Part B4 Drainage and Water Quality of Port Stephens Council DCP, the stormwater management strategy has considered the following items which will be discussed in the following sections of this report:

- Stormwater Drainage Plan
- On-site detention / on-site infiltration
- Water Quality

Stormwater Drainage Plan

Plans detailing the stormwater system design intent of the proposed development is appended to the rear of this report.

The Northern catchment is drained via a pit and pipe network that conveys the captured stormwater to an on-site infiltration trench, designed to infiltrate the 1% AEP event. Overflow from the trench is proposed to connect to the existing internal drainage system that overflows into an existing creek to the north of the site that ultimately drains to Salamander Bay.

The southern catchments are also drained via a pit and pipe network to proposed on-site detention tanks, designed to attenuate post developed flows for the development footprint back to greenfield flows. Outflow from the tanks connect to the existing internal drainage network that ultimately discharge to the Council network in Fleet St. Infiltration was not proposed for the southern catchment given the limited footprint available (outside of buildings and existing roads) and the high groundwater level. New units are proposed to have individual rainwater reuse tanks that will be reticulated both internally for toilet flushing and laundry and externally for landscape irrigation and hardstand washdown.

Stormwater Quantity

Council guidelines require on-site stormwater detention and infiltration to limit post developed flow to predeveloped volume and flow rates for all storm events up to and including the 1% AEP storm event. Runoff from the proposed development was modelled using the runoff routing software DRAINS. A lumped-node approach was taken to simulate the infiltration and detention volumes required to adequately satisfy the Council guidelines. As directed by Council, the Northern catchment of the development is to be directed to an infiltration system via pit and pipe network, where all design storms up to the 1% AEP critical event are to be infiltrated with no overflow. A FoS of 3 was used for the infiltration values, inline with advice provided by geotechnical report 754-NTLGE307333-AB. Applying the FoS, an infiltration rate of 3.67×10^{-5} m/sec was used for the infiltration trench within the model. Figure 2 displays the schematic of the DRAINS model.

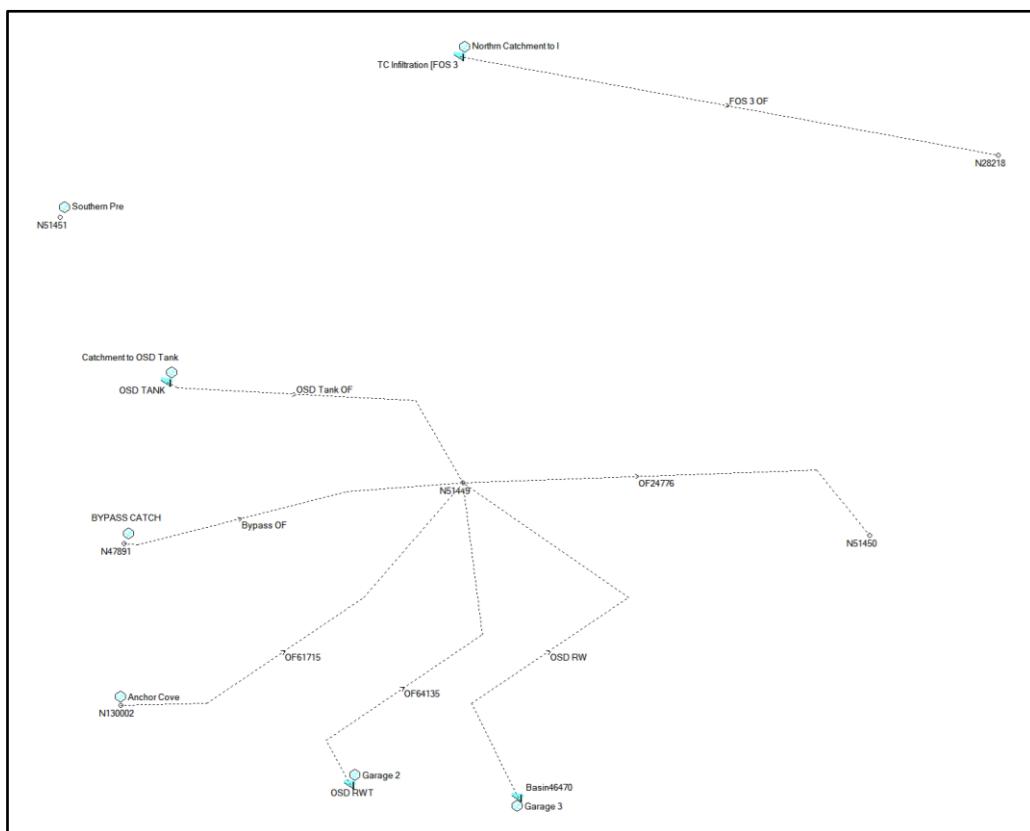


Figure 2 - DRAINS model schematic

The Continuing Loss Hydrological model in DRAINS was used to generate runoff hydrographs for the pre and post developed site. Data from the Bureau of Meteorology and ARR Data Hub was used to generate design storms. Runoff Parameters were selected to replicate the site conditions that will be present in the post-developed case, and that which currently occur in the pre-developed case. A summary of parameters used for the model are shown below:

Impervious Area Initial Loss	= 0 mm
Impervious Area Continuing Loss	= 0 mm/hr
Pervious Area Initial Loss	= 6.3 mm
Pervious Area Continuing Loss	= 0.84 mm/hr

Storm durations ranging from 5 minutes to 72 hours were investigated for each of the design storm events.

The southern catchment contains several OSD systems including a 19m³ underground detention tank, one 6kL and two 3kL above ground combined reuse/OSD tanks. The predeveloped scenario for the southern catchment has been broken up into the new central area draining to the OSD Tank 1 [South 1] and the new garages and alterations to the very southern portion of the site [south 2].

The South 1 catchment's predeveloped flowrates are based on a greenfield site. The South 2 catchment has been modelled to include the existing hardstand within the development footprint as this the area is currently developed and the works are not a significant change to the current drainage regime. Refer to the predeveloped Drains catchment plan for further details. A comparison between the pre-development and post-development peak flows from the site for the critical storm duration for each return interval up to and including the 1% AEP is presented in Table 1.

Table 1 - DRAINS Results Southern Catchment

AEP	Pre-Flowrate (m ³ /s)		Post-Flowrate (m ³ /s)	
	South 1	South 2	South 1	South 2
1%	0.369	0.147	0.315	0.130
2%	0.308	0.126	0.260	0.113
5%	0.242	0.103	0.200	0.093
10%	0.197	0.086	0.167	0.078
20%	0.144	0.064	0.133	0.061

The infiltration trench is designed with a storage depth of 0.7m above the infiltration floor at RL 7.0m AHD, which is approximately 0.5m above the estimated ground water level (GWL) of RL 6.5m AHD from the infiltration testing in report 754-NTLGE307333-AB. It is acknowledged the buffer to the GWT is less than the 1m specified by PSC, however it is noted that onsite potholing was undertaken on 21.02.23 and the water level was intercepted at RL5.51 as shown in Figure 3. The full plan is appended to this report. This level is significantly lower than the aforementioned RL6.5AHD nominated within the infiltration testing report. As such it is recommended that further long term GWL monitoring be undertaken during the detailed design and if required the trench layout/depth can be adjusted.

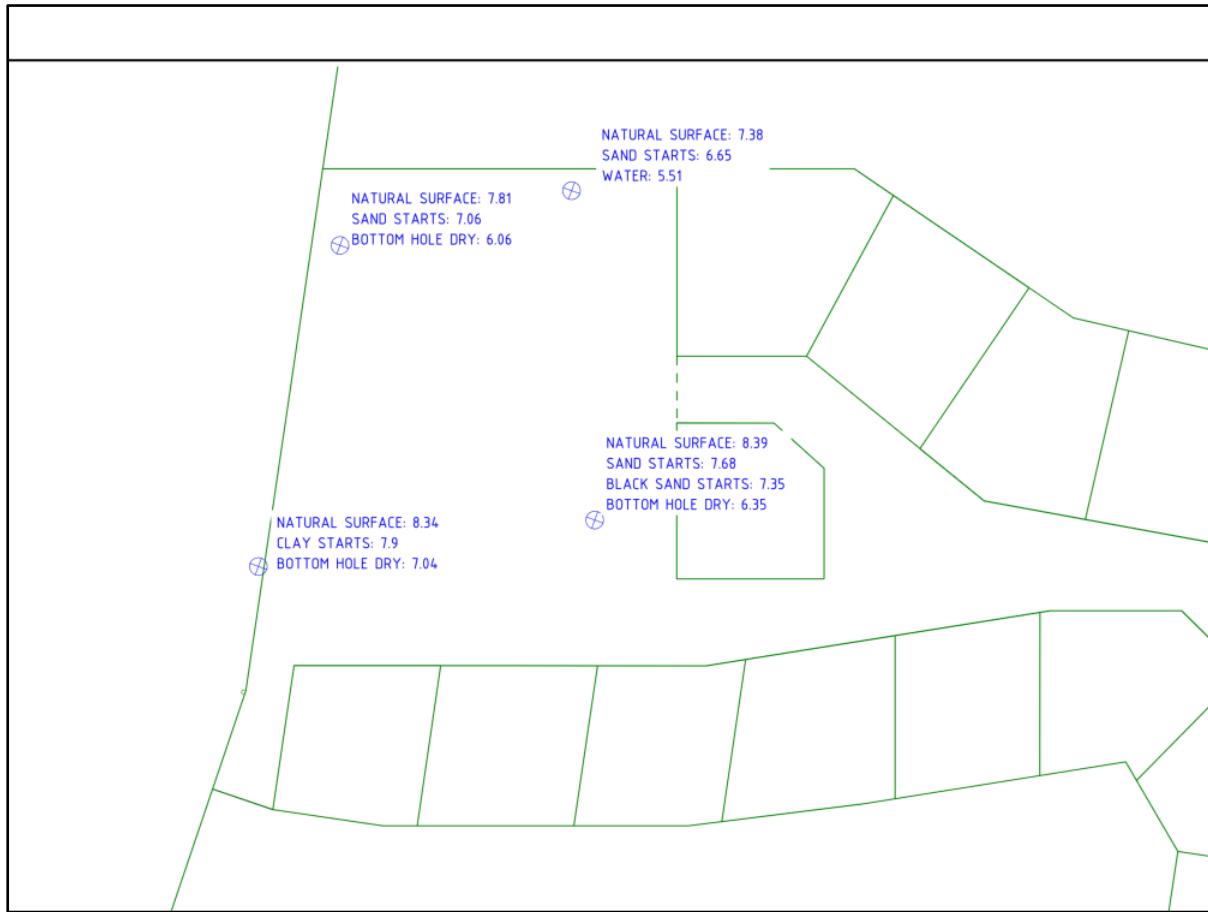


Figure 3. Onsite potholing results

If a GWL of RL6.5m is adopted, to provide the 1m buffer, it will be infeasible to infiltrate the 1% AEP design storm, given the available footprint is limited. The Total Infiltration storage currently provided is 420m³.

The below ground OSD tank contained within the Southern catchment is designed with a maximum 0.80m detention depth. Storm events are to be controlled via a 250mm low flow orifice plate with an invert at the base of the tank and an internal weir elevated 0.6m from the base of the tank. Flows are to discharge from the tank via an outlet pipe into the existing pit and pipe network. The 6kL above ground combined OSD/Reuse tank is designed with a 70mm low flow orifice outlet and the two 3kL above ground combined OSD/Reuse tanks are designed with a 50mm low flow orifice outlet.

The results in Table 1 show that a proposed detention strategy should effectively attenuate runoff up to the 1% AEP event and therefore effectively mitigate the effect of the development on stormwater quantity in accordance with the DCP. The proposed infiltration trench effectively infiltrates all storm events up to and including the 1% AEP. The DRAINS model is available upon request.

Stormwater Quality

In order to minimise adverse impacts upon the ecology of downstream watercourses and achieve Council's stormwater pollutant reduction targets, stormwater treatment devices are proposed to be incorporated into the design of the development. The adopted nutrient and pollution targets are summarised in Table 1:

Table 1 - Required Water Quality Targets

Pollutant Criteria	Required Reduction Target (%)
Total Suspended Solids (TSS)	90
Total Phosphorous (TP)	60
Total Nitrogen (TN)	45
Gross Pollutants	90

The performance of the proposed stormwater management strategy was assessed against these targets using the conceptual design software MUSIC (Version 6). The MUSIC model was developed using parameters recommended in the document “NSW MUSIC Modelling Guidelines” (WBM, 2015), Council Guidelines and Port Stephens Council MUSIC Link.

The total catchment area was split into sub-catchments representing the areas draining to the different treatment devices. A schematic of the MUSIC model is provided in Figure 4.

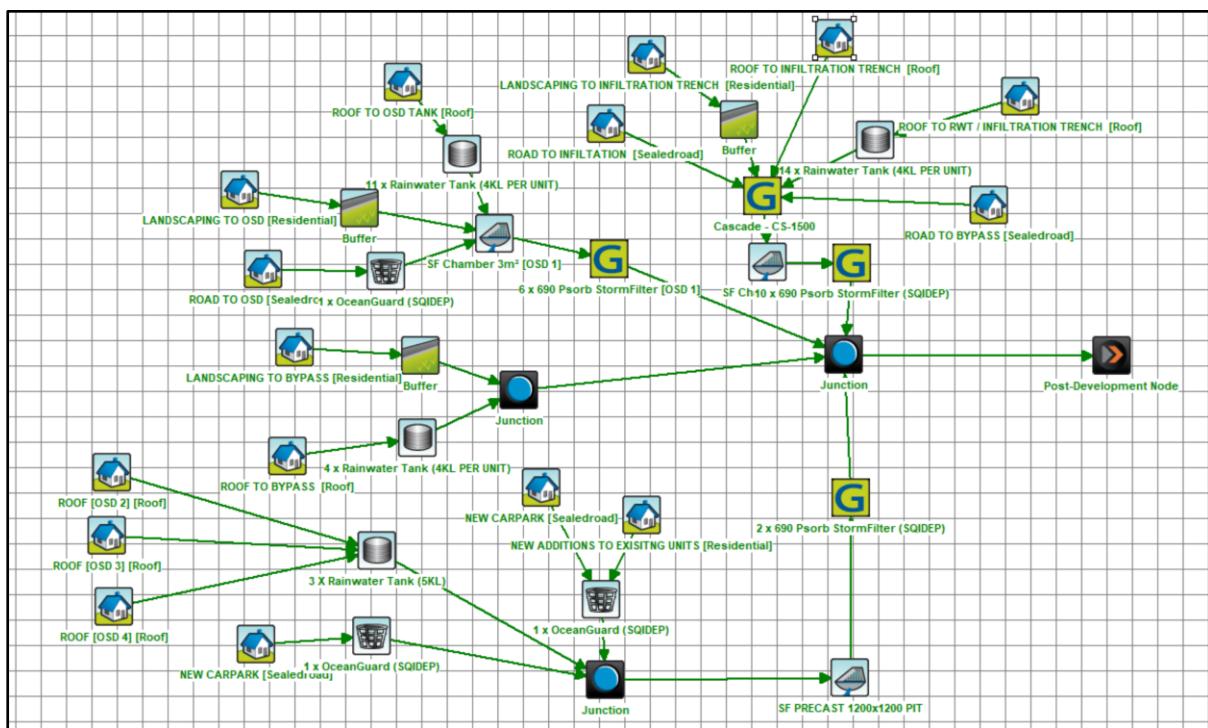


Figure 4 - MUSIC Model Schematic

The following is a summary of the water quality treatment devices that have been utilised in the proposed treatment train.

- **Rainwater Harvesting Tank** – Runoff from roof areas are to be directed to rainwater harvesting tanks. The tanks are to be fitted with a proprietary first-flush device which will effectively remove dead insects, bird and animal droppings and concentrated tannic acids from the stormwater system. The rainwater tank will also provide secondary treatment by acting as an initial sediment trap, collecting suspended solids and the nutrients attached to those sediments. 4kL rainwater tanks are to be provided for each residential unit, and a 5kL rainwater tank is to be provided for the each of the garage units located within Anchor Cove. The re-use rates of the rainwater tanks have been modelled as per WSUD Technical Guidelines. Water captured in the rainwater tanks is expected to be reused for internal re-use and external irrigation.

- **Sediment Trap** – Captured runoff is directed into a sediment trap containing the filter cartridges. The sediment trap further assists with the collecting of suspended solids and any attached nutrients.
- **Proprietary Filter Cartridges** (e.g. Ocean Protect Psorb Stormfilter) – Water within the sediment trap is further treated with siphon actuated cartridges that passes water evenly through a filter media to efficiently capture total suspended solids, hydrocarbons, nutrients and soluble heavy metals.
- **Proprietary Pit Filter Insert** (e.g. Ocean Protect Ocean Guard) – Runoff collected via surface inlet pits will be passed through a proprietary pit filter insert. The filter inserts will prevent ingress of gross pollutants as well as removed small amounts of nutrients and fine sediments attached to larger items captured.
- **Proprietary Gross Pollutant Trap** (e.g. Ocean Protect Cascade) – Runoff collected prior to entering the infiltration trench will be conveyed via a proprietary GPT. The GPT will prevent ingress of gross pollutants as well as removed small amounts of nutrients and fine sediments attached to larger items captured.

A small portion of the proposed road will bypass the treatment train [area b2, approximately 145m²]. The GPT and Stormfilter cartridges that treat the runoff prior to entering the infiltration trench will capture an additional existing pavement area that is greater than the 145m² that is bypassing the system [over 200m³]. As such the 145m² of catchment [road to bypass] has been added to the treatment train at the GPT to reflect the conditions that will be present onsite.

Table 3 presents the reduction in pollutants for an indicative treatment train as outlined above.

Table 3 - MUSIC Model Results

	Source Loads (kg/yr)	Residual Loads (kg/yr)	% Achieved Reduction	% Target Reduction
TSS	1210	103	91.5	90
TP	2.73	0.68	75.1	60
TN	23.7	7.99	66.3	45
GP	245	15.1	93.8	90

Based on the results shown in Table 3, the proposed water quality treatment system will meet the design intent and reduction targets. An electronic copy of the MUSIC model can be provided upon request, and the MUSIC Link report is appended to the rear of this report.

In conclusion, the stormwater management plan prepared by Northrop demonstrates that the pollution reduction targets by Council can be achieved by implementing the treatment devices described above onsite. Post development flows for the southern catchment should be contained and reduced to pre-development flowrates with the proposed and detention storage tanks. For the northern catchment flows up to the 1% AEP event will be infiltrated via a proposed infiltration trench.

The catchment plans for both the DRAINS and MUSIC modelling have been appended to the rear of this report for context.

We trust this meets your expectations. Should you have any queries, please feel free to contact the undersigned.

Yours Sincerely,



Thomas van Koeverden
Civil Engineer
B.E. Civil (Hons) | MIEAust

Limitation Statement

Northrop Consulting Engineers Pty Ltd (Northrop) has been retained to prepare this report based on specific instructions, scope of work and purpose pursuant to a contract with its client. It has been prepared in accordance with the usual care and thoroughness of the consulting profession for the use by Wanda Beach Estate Pty Ltd. The report is based on generally accepted practices and standards applicable to the scope of work at the time it was prepared. No other warranty, express or implied, is made as to the professional advice included in this report.

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WANDA BEACH ESTATE

FLEET STREET, SALAMANDER BAY NSW 2317
CIVIL ENGINEERING PACKAGE



IMAGE SOURCE: NEARMAPS

LOCALITY PLAN

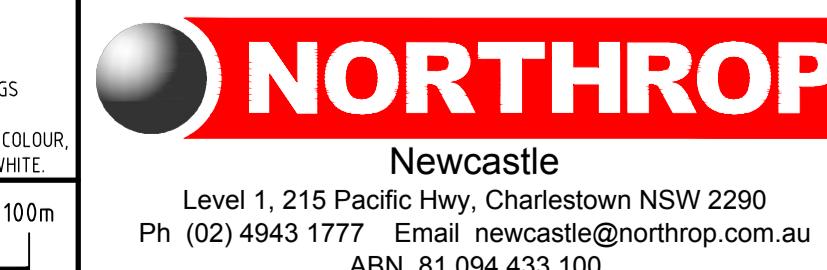
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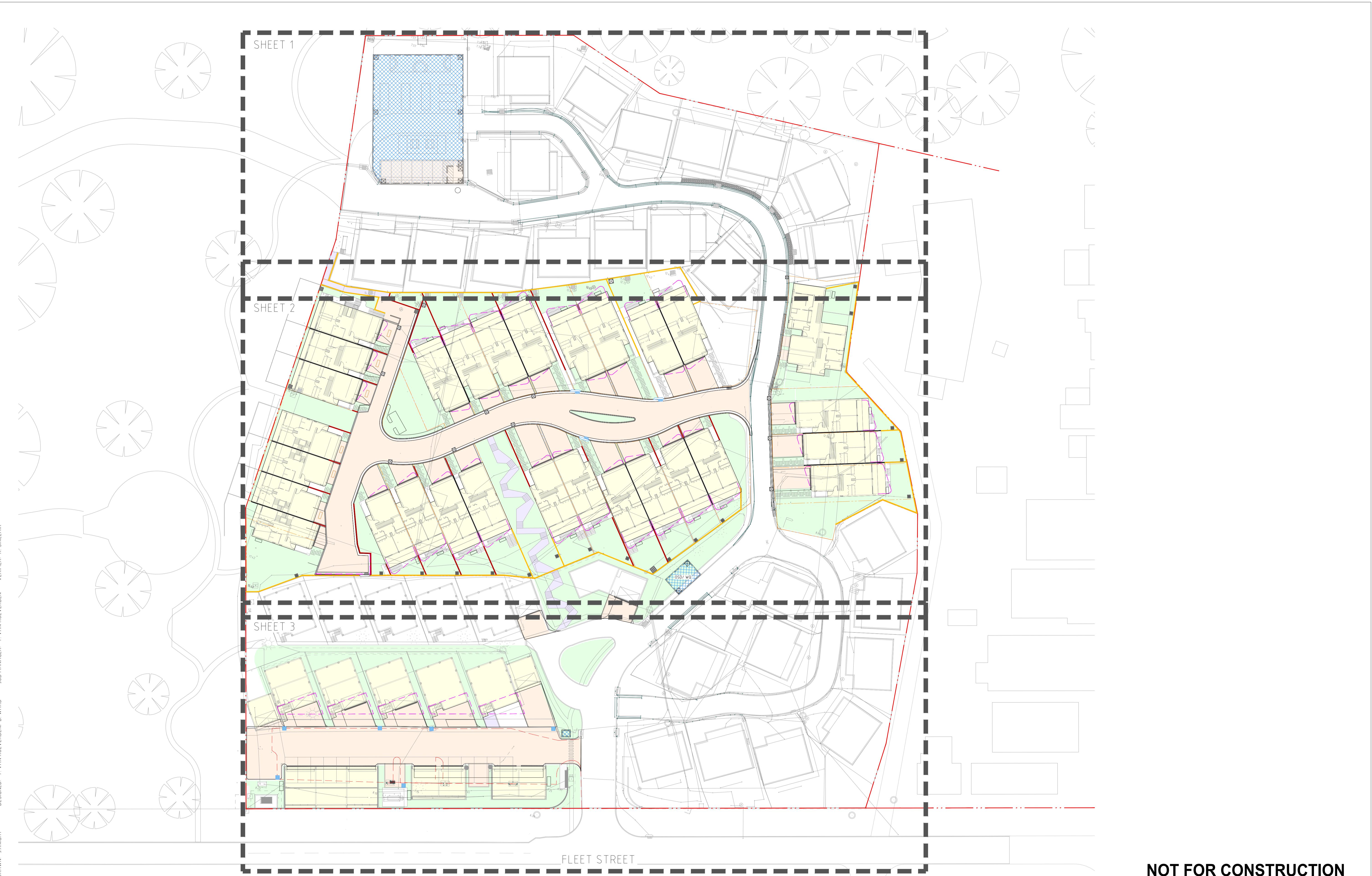
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DA-C02.10	EROSION AND SEDIMENT CONTROL DETAILS
DA-C03.01	CUT AND FILL PLAN
DA-C04.01	CIVIL WORKS PLAN - SHEET 1
DA-C04.02	CIVIL WORKS PLAN - SHEET 2
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DA-C09.01	CIVIL DETAILS

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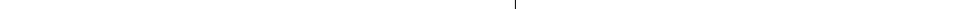
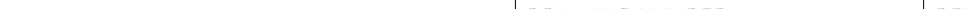
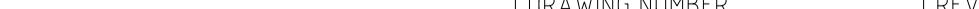
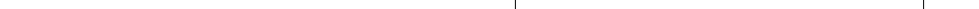
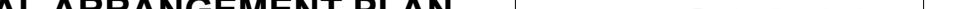
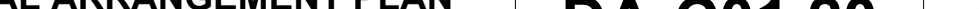
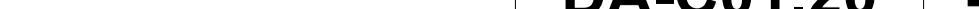
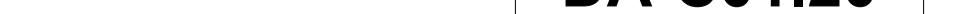
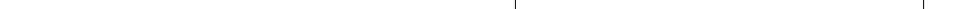
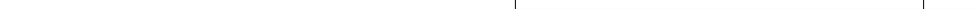
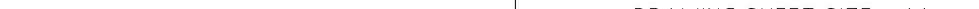
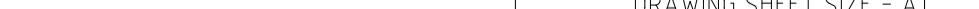
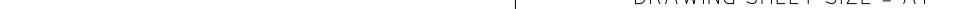
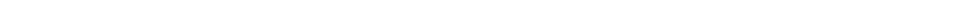
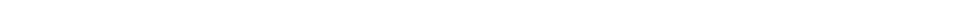
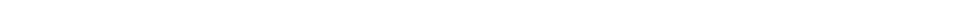
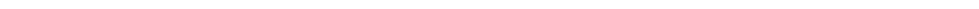
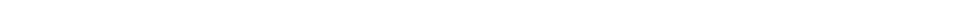
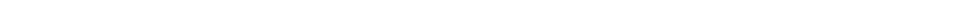
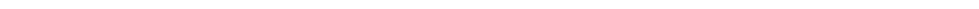
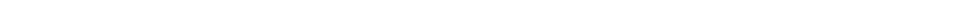
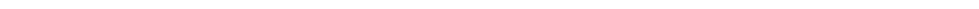
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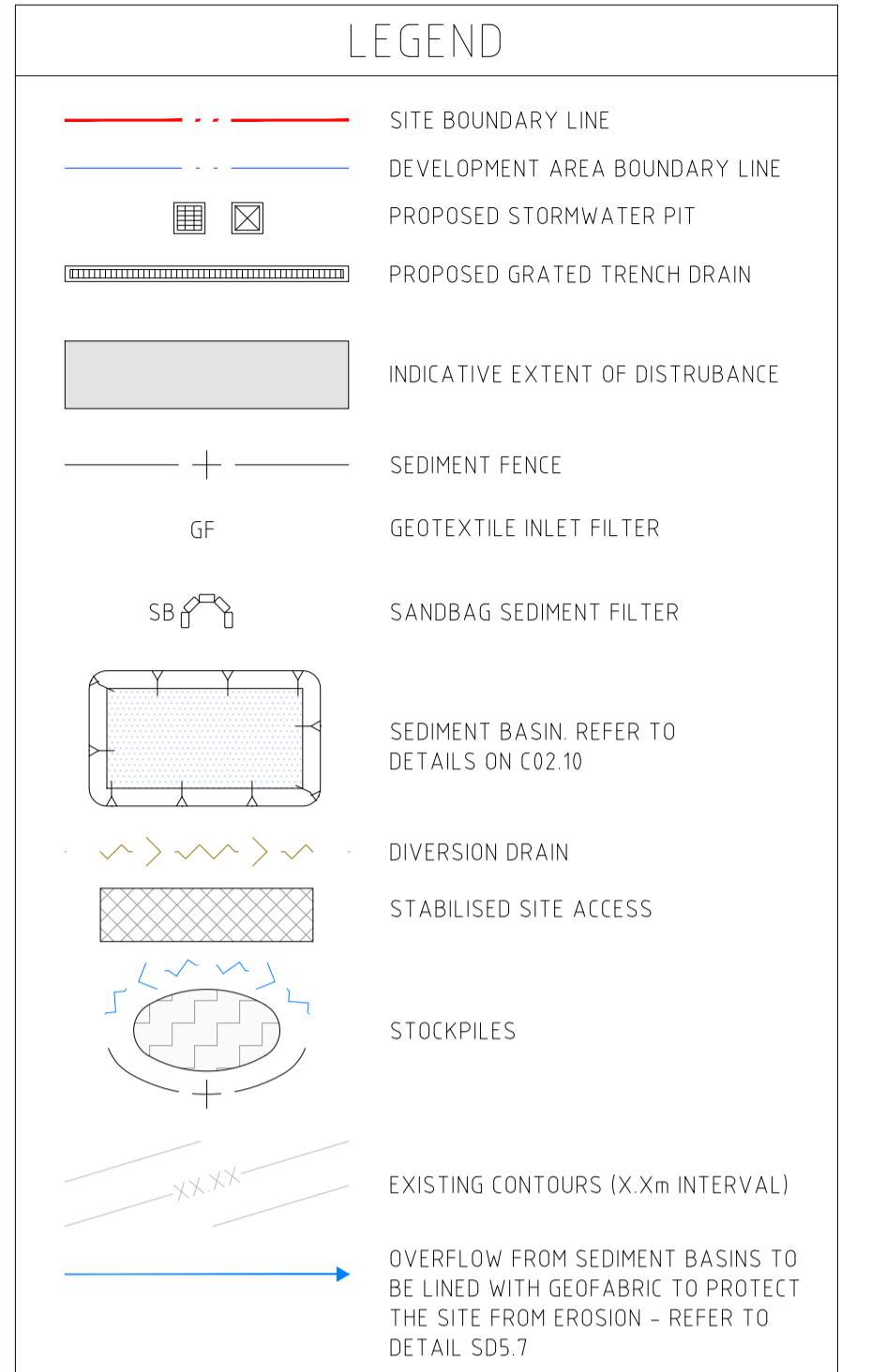
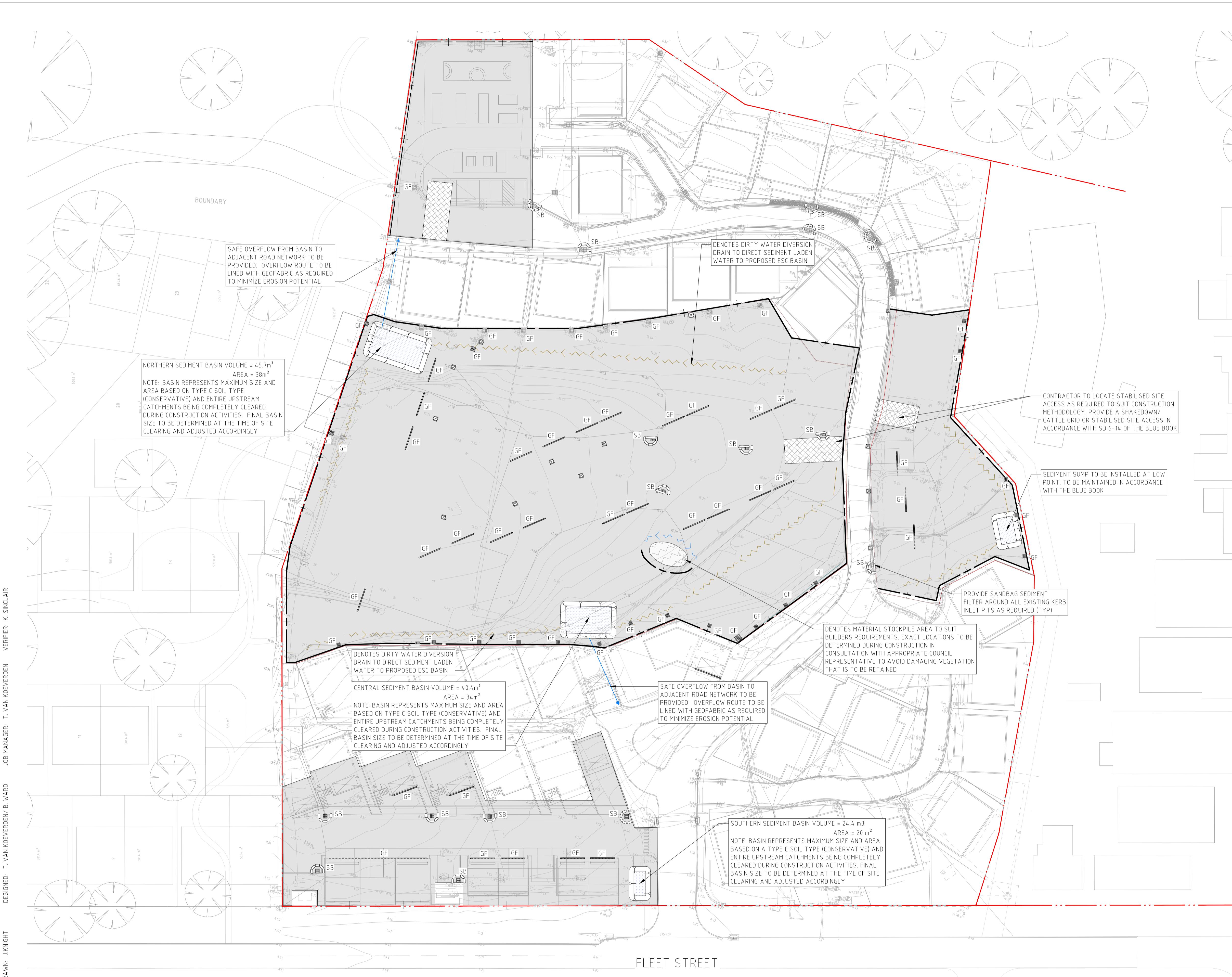


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NOTE

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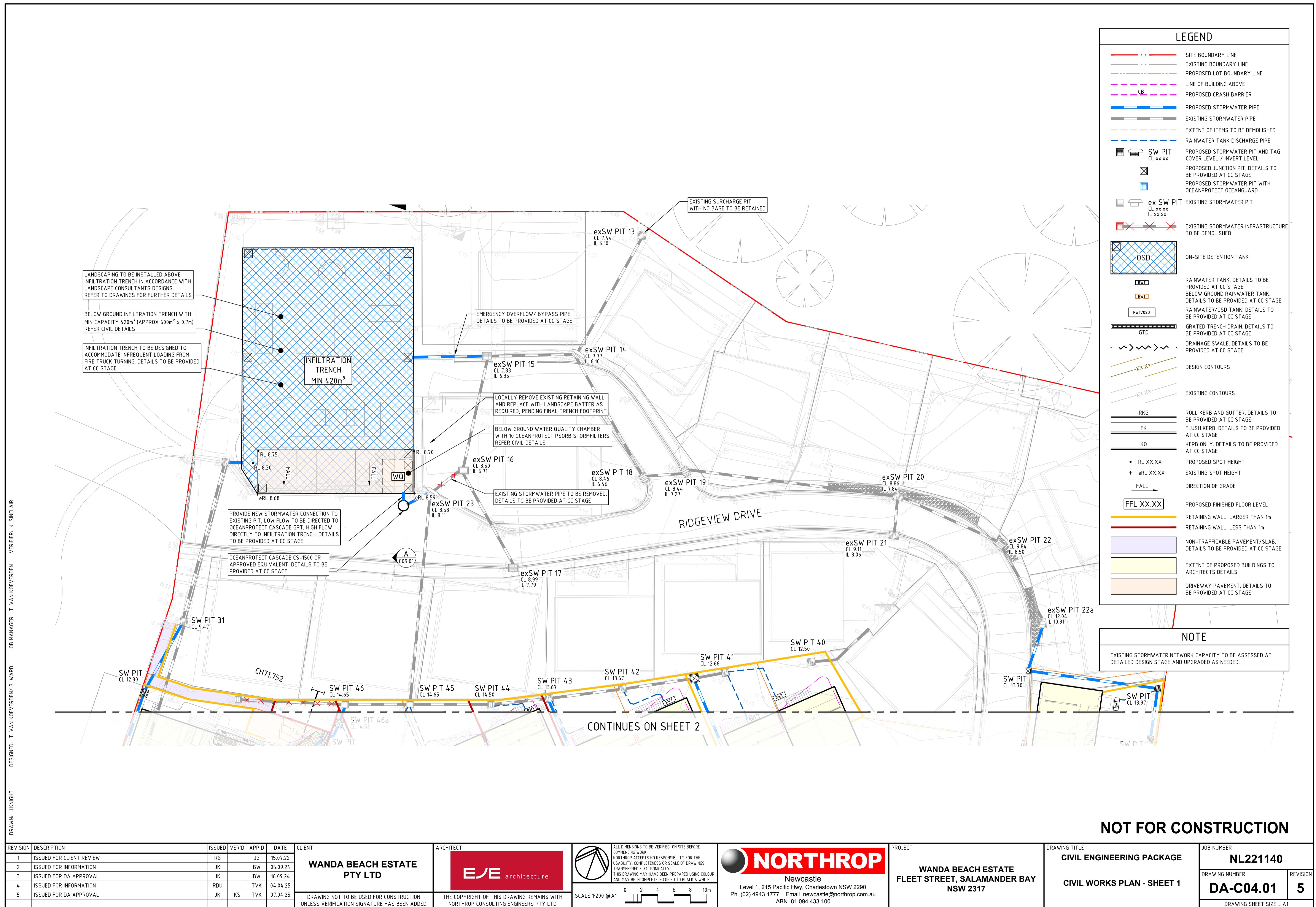
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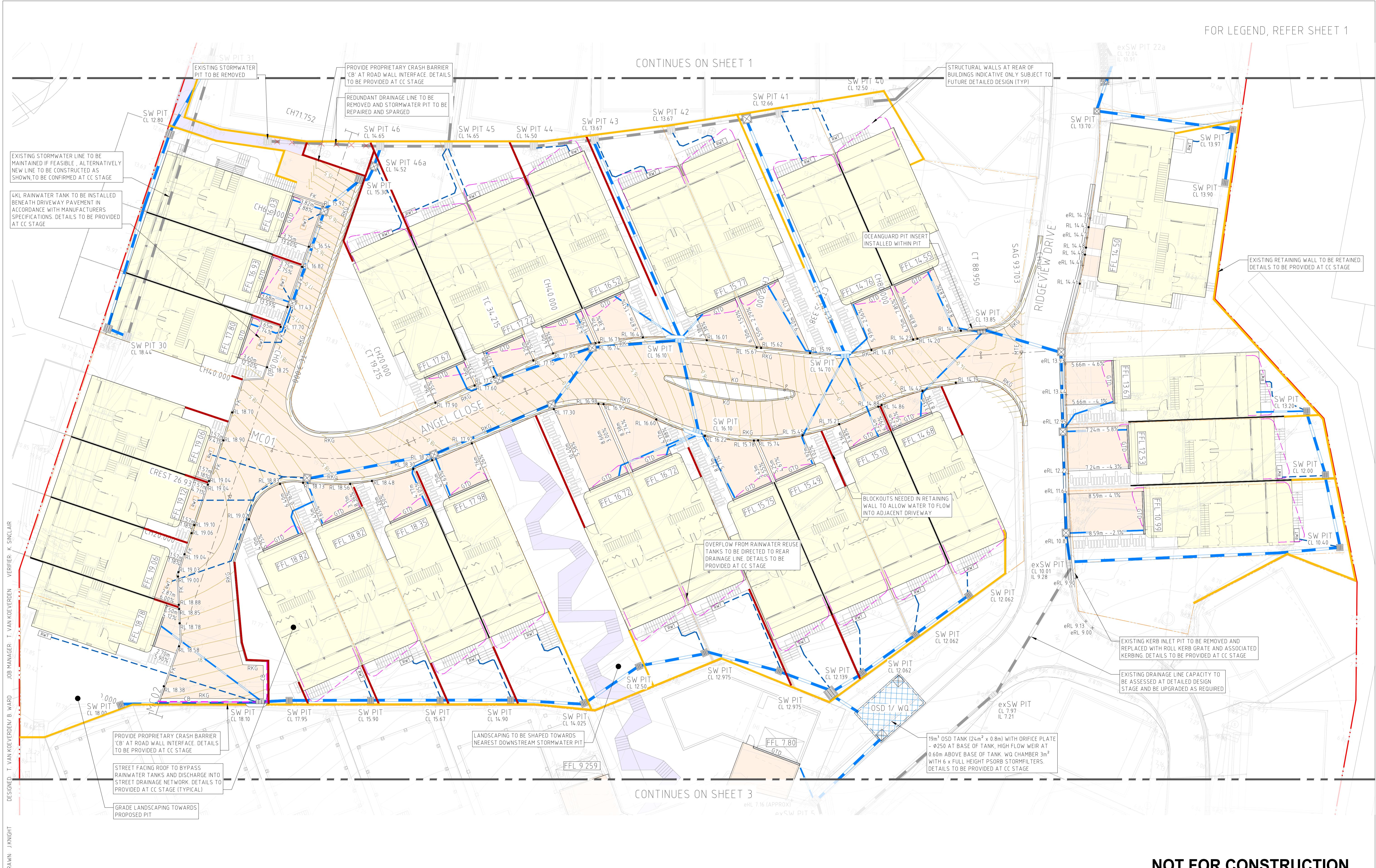
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FOR LEGEND, REFER SHEET 1



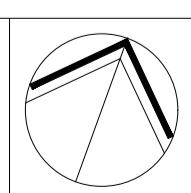
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3	ISSUED FOR INFORMATION	RDU	TVK	04.04.25		
4	ISSUED FOR DA APPROVAL	JK	KS	TVK	07.04.25	

WANDA BEACH ESTATE
PTY LTD



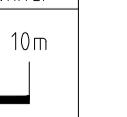
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ABN 81 094 433 100

PROJECT
WANDA BEACH ESTATE
FLEET STREET, SALAMANDER BAY
NSW 2317

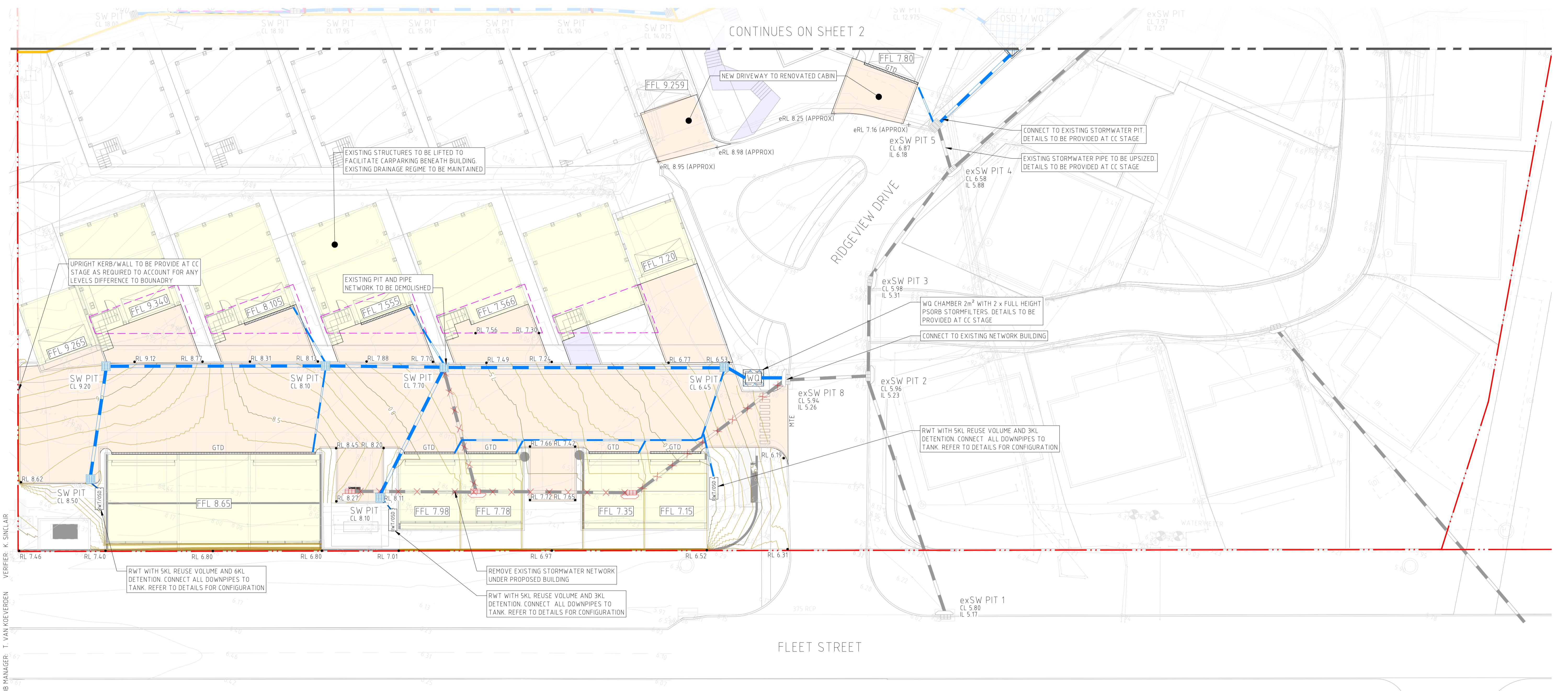
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CIVIL ENGINEERING PACKAGE
CIVIL WORKS PLAN - SHEET 2

JOB NUMBER
NL221140

DRAWING NUMBER
DA-C04.02

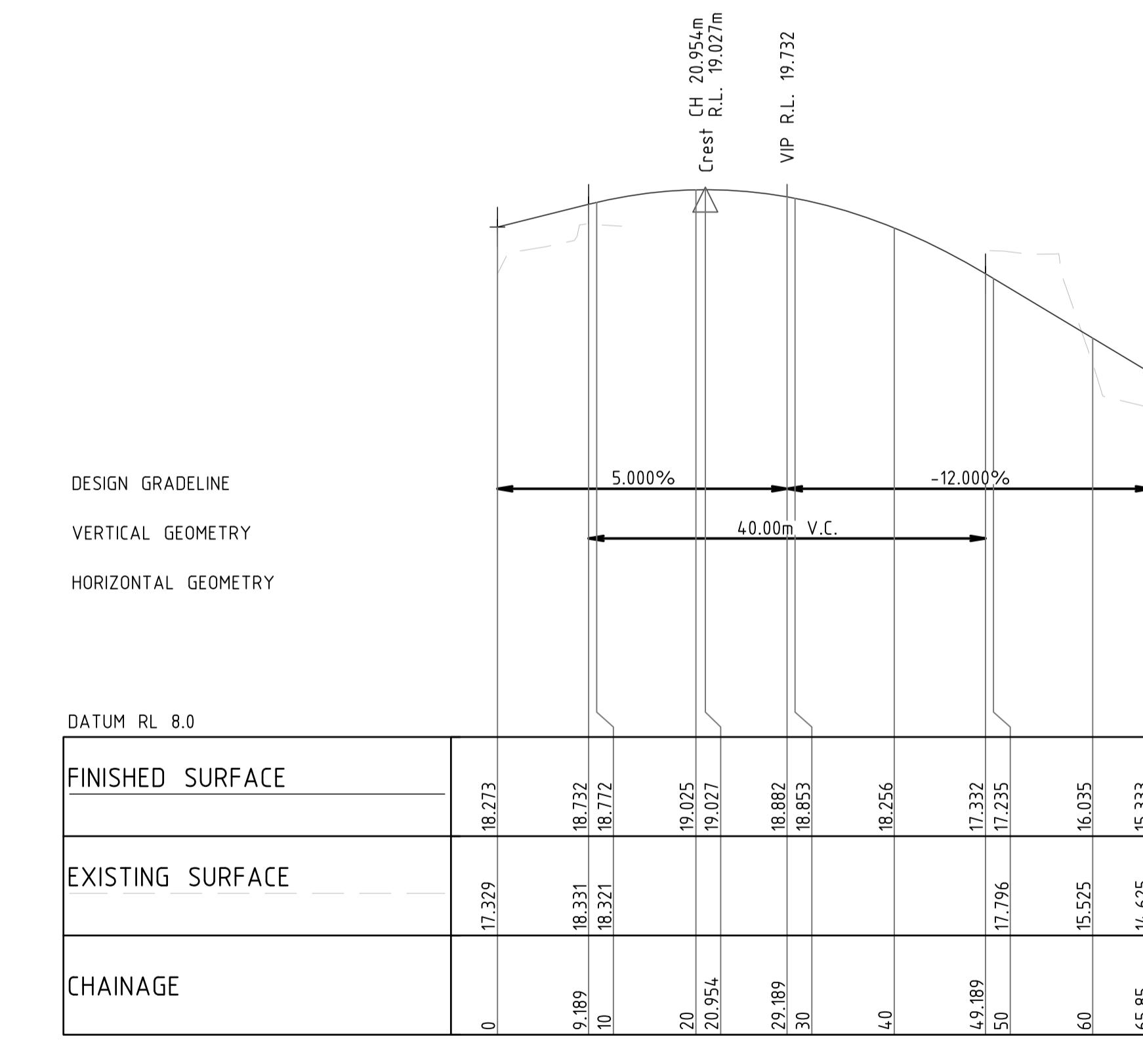
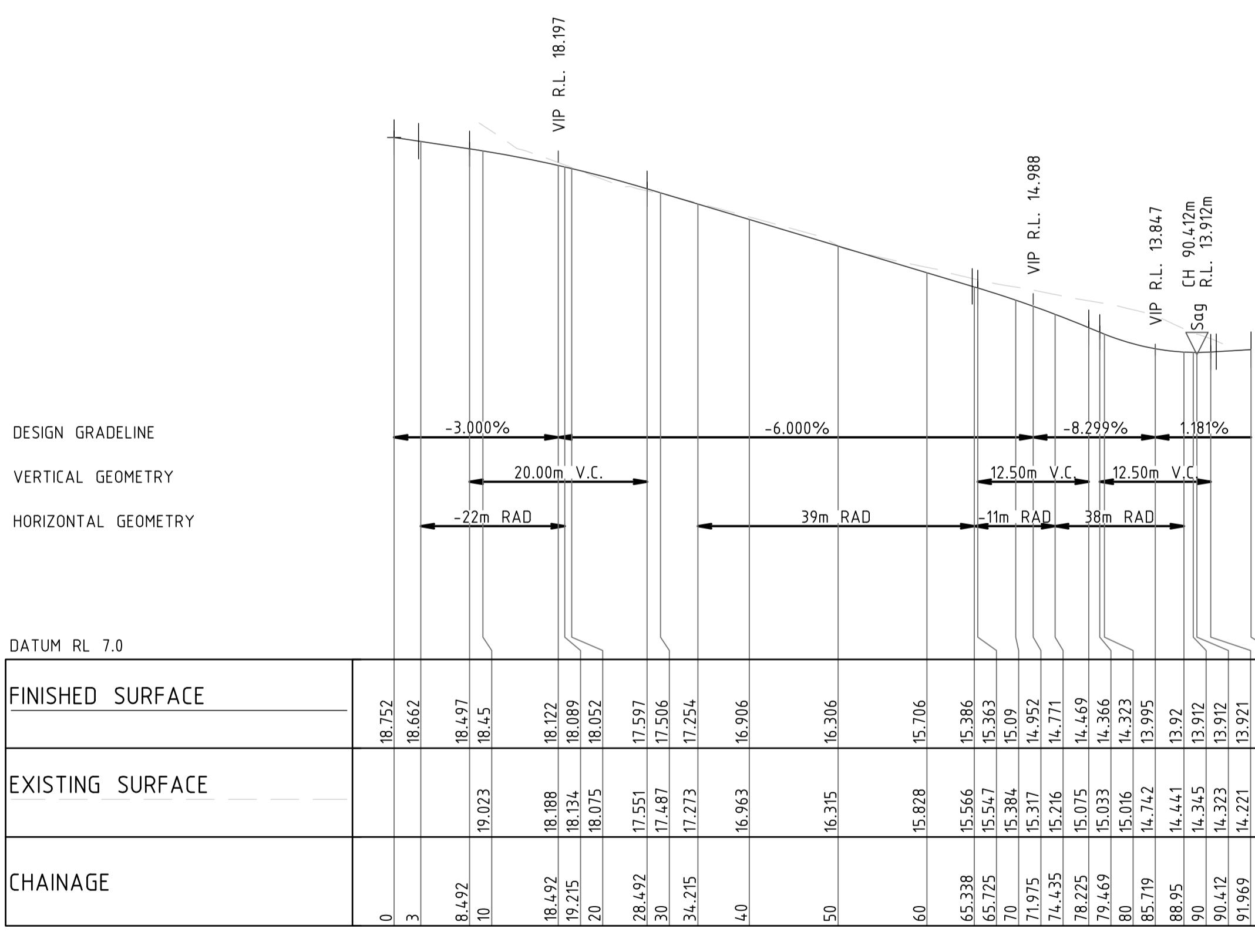
REVISION
4

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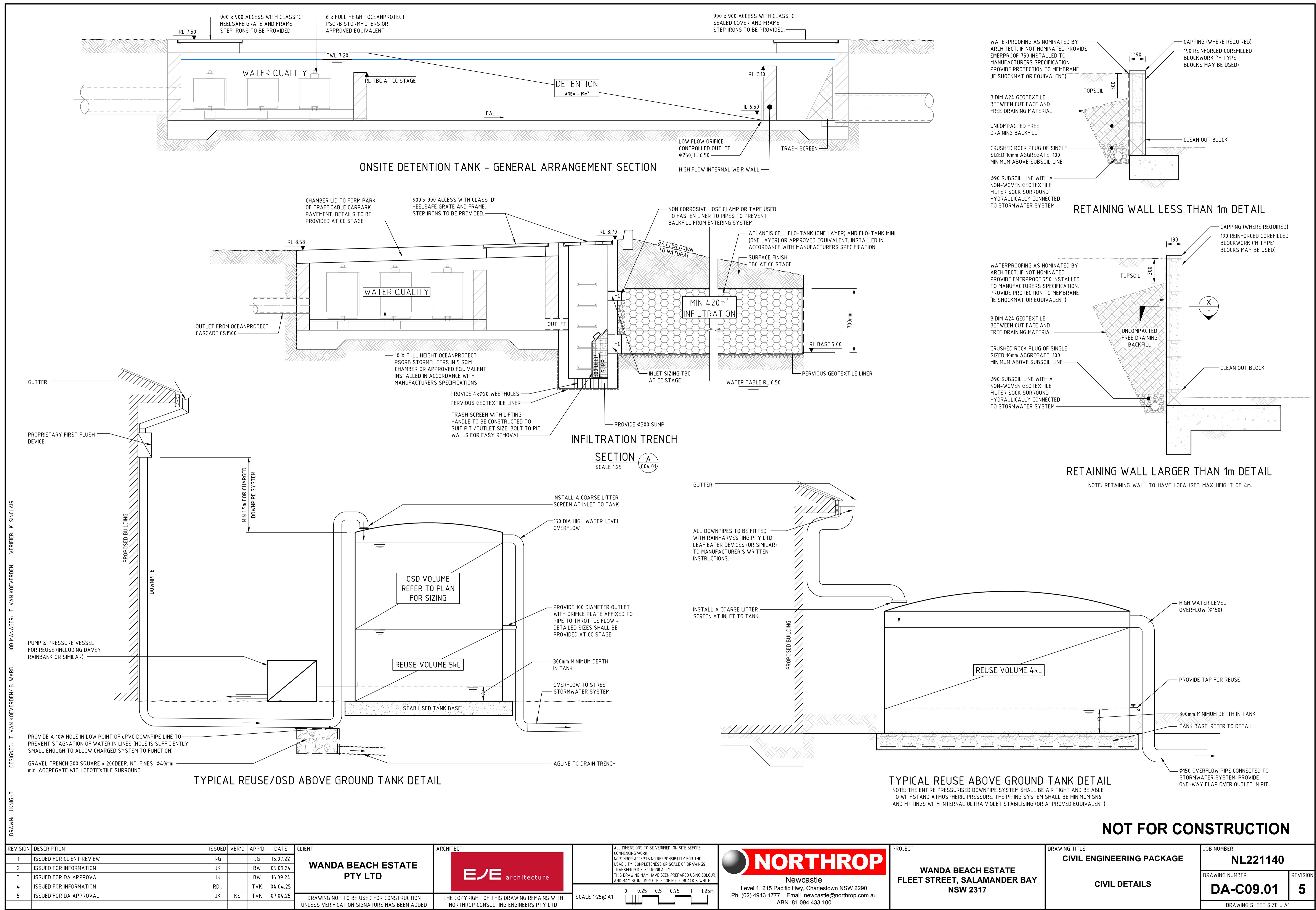


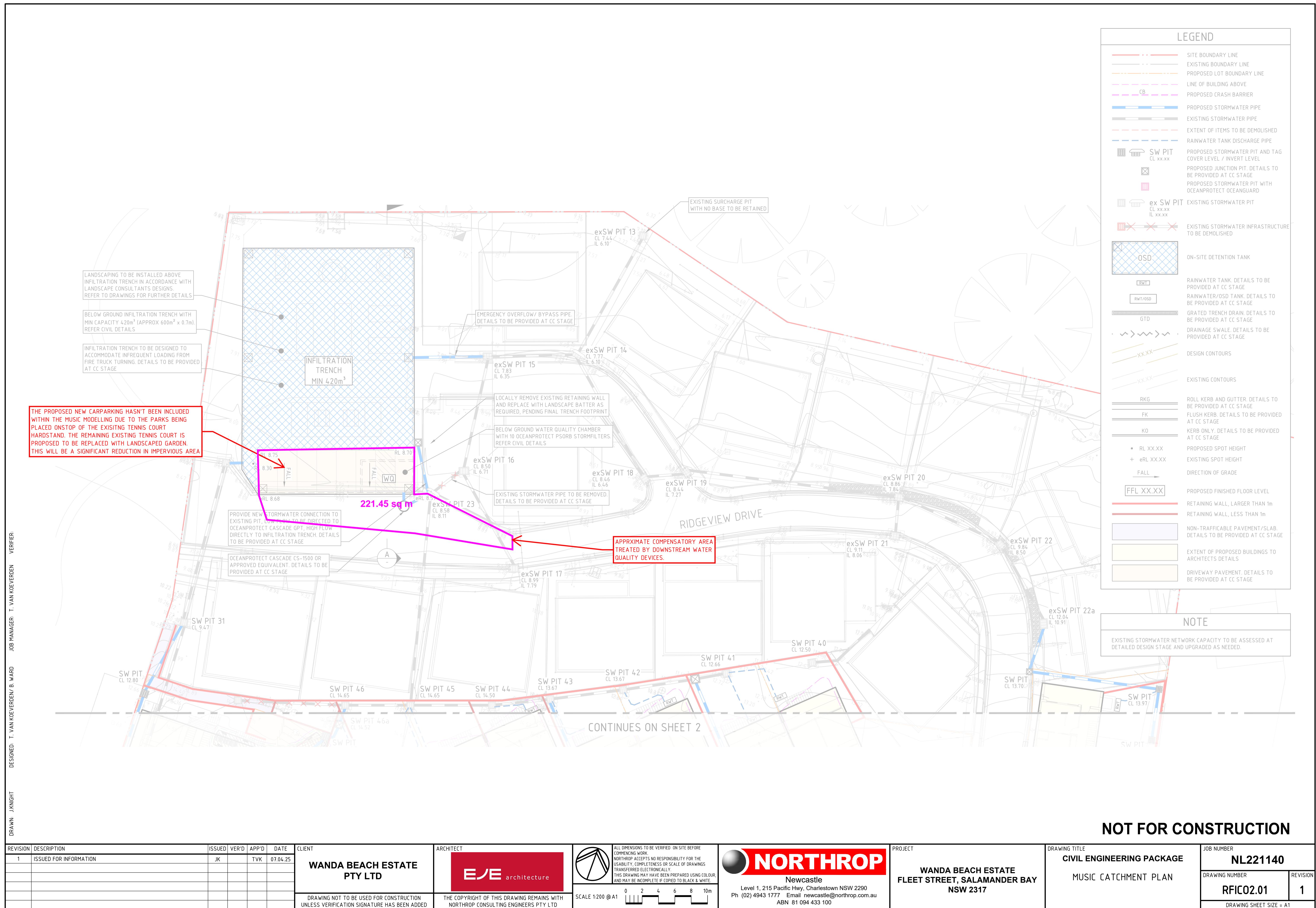
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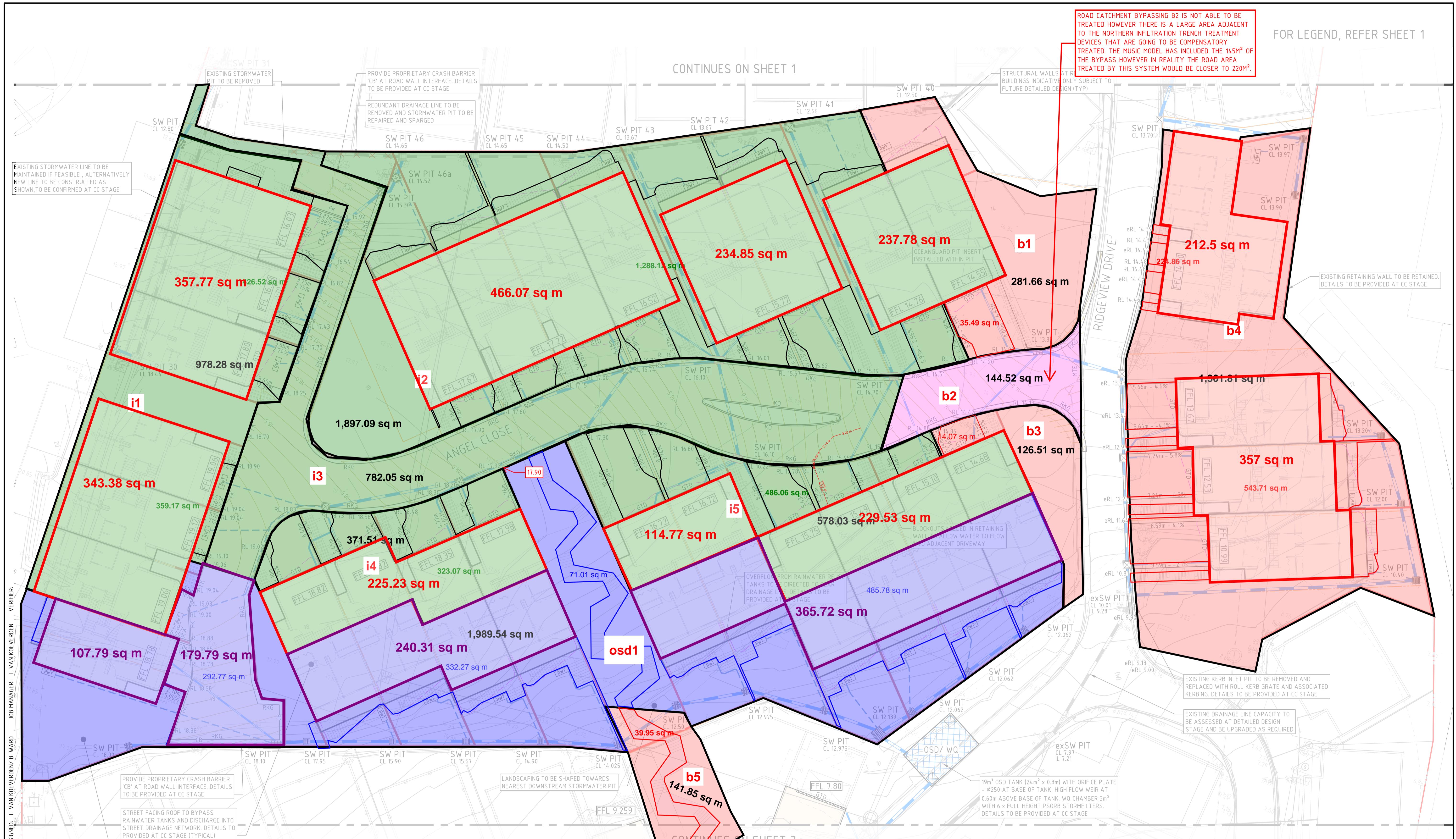
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2	ISSUED FOR DA APPROVAL	JK		BW	16.09.24						
3	ISSUED FOR INFORMATION	RDU		TVK	04.04.25						
4	ISSUED FOR DA APPROVAL	JK	KS	TVK	07.04.25						
						DRAWING NOT TO BE USED FOR CONSTRUCTION UNLESS VERIFICATION SIGNATURE HAS BEEN ADDED	THE COPYRIGHT OF THIS DRAWING REMAINS WITH NORTHROP CONSULTING ENGINEERS PTY LTD	SCALE 1:200 @ A1	0 2 4 6 8 10m	DRAWING NUMBER	
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										DA-C04.03	4
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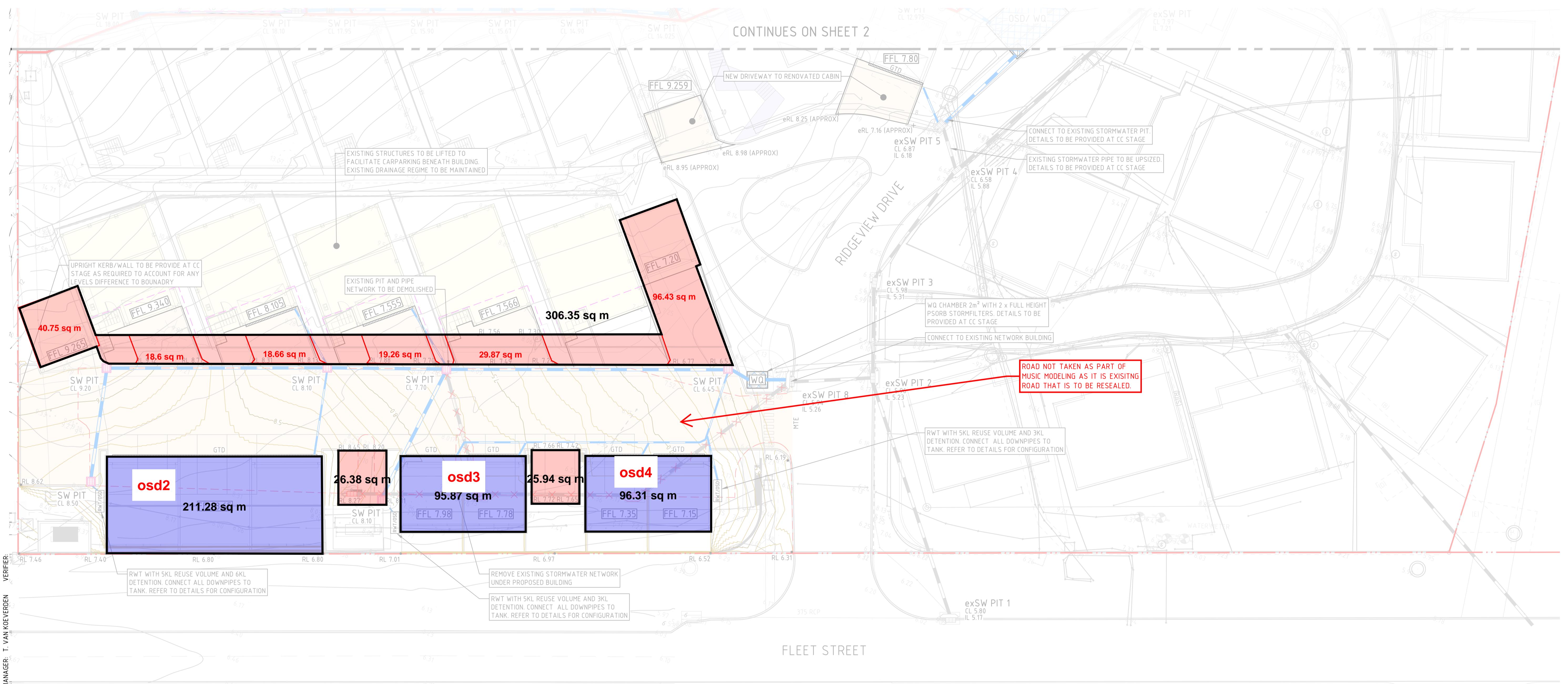
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1	ISSUED FOR INFORMATION	JK	TVK	07.04.25		WANDA BEACH ESTATE PTY LTD	EJE architecture			NORTHROP	CIVIL ENGINEERING PACKAGE	NL221140
DRAWING NOT TO BE USED FOR CONSTRUCTION UNLESS VERIFICATION SIGNATURE HAS BEEN ADDED												
										WANDA BEACH ESTATE FLEET STREET, SALAMANDER BAY NSW 2317	MUSIC CATCHMENT PLAN	
											DRAWING NUMBER	RFIC02.03
											REVISION	1
											DRAWING SHEET SIZE	A1

MUSIC-link Report

Project Details		Company Details			
Project:	NL221140	Company:	Northrop Consulting Engineers		
Report Export Date:	4/04/2025	Contact:	Thomas van Koevorden		
Catchment Name:	NL221140 - DA MUSIC Model [2]	Address:	215 Pacific Highway, Charlestown, NSW 2290		
Catchment Area:	1.01ha	Phone:	49431777		
Impervious Area*:	80.06%	Email:	tkoevorden@northrop.com.au		
Rainfall Station:	VILLIUMTOWN RAAF - Station 061078 - Zone D				
Modeling Time-step:	6 Minutes				
Modeling Period:	1/01/1998 - 31/12/2007 11:54:00 PM				
Mean Annual Rainfall:	1351mm				
Evapotranspiration:	1394mm				
MUSIC Version:	6.3.0				
MUSIC-link data Version:	6.35				
Study Area:	Seaham				
Scenario:	Default Catchment - Sandy soils				

* takes into account area from all source nodes that link to the chosen reporting node, excluding Import Data Nodes

Treatment Train Effectiveness		Treatment Nodes		Source Nodes	
Node: Post-Development Node	Reduction	Node Type	Number	Node Type	Number
Flow	20.1%	Rain Water Tank Node	4	Urban Source Node	16
TSS	91.5%	Sedimentation Basin Node	3		
TP	75.1%	Buffer Node	3		
TN	66.3%	Generic Node	4		
GP	93.8%	GPT Node	3		

Comments
Proprietary nodes used in accordance with manufacturers recommended inputs.

Passing Parameters		Parameter	Min	Max	Actual
Node Type	Node Name				
Buffer	Buffer	Proportion of upstream impervious area treated	None	None	1
Buffer	Buffer	Proportion of upstream impervious area treated	None	None	1
Buffer	Buffer	Proportion of upstream impervious area treated	None	None	1
GPT	1 x OceanGuard (SOI/DEP)	Hi-flow bypass rate (cum/sec)	None	99	0.02
GPT	1 x OceanGuard (SOI/DEP)	Hi-flow bypass rate (cum/sec)	None	99	0.02
GPT	1 x OceanGuard (SOI/DEP)	Hi-flow bypass rate (cum/sec)	None	99	0.02
Post	Post/Development Node	% Load Reduction	None	None	20.1
Post	Post/Development Node	GP % Load Reduction	90	None	93.8
Post	Post/Development Node	TN % Load Reduction	45	None	66.3
Post	Post/Development Node	TP % Load Reduction	60	None	75.1
Post	Post/Development Node	TSS % Load Reduction	90	None	91.5
Rain	11 x Rainwater Tank (4KL PER UNIT)	% Reuse Demand Met	None	None	33.46
Rain	14 x Rainwater Tank (4KL PER UNIT)	% Reuse Demand Met	None	None	50.34
Rain	3 X Rainwater Tank (5KL)	% Reuse Demand Met	None	None	33.1893
Rain	4 x Rainwater Tank (4KL PER UNIT)	% Reuse Demand Met	None	None	42.96
Sedimentation	SF Chamber 3m ² (GSD 1)	High Flow Bypass Out (ML/yr)	None	None	0
Sedimentation	SF Chamber 5m ²	% Reuse Demand Met	None	None	0
Sedimentation	SF Chamber 5m ²	High Flow Bypass Out (ML/yr)	None	None	0
Sedimentation	SF PRECAST 1200x1200 PIT	% Reuse Demand Met	None	None	0
Sedimentation	SF PRECAST 1200x1200 PIT	High Flow Bypass Out (ML/yr)	None	None	0
Urban	LANDSCAPING TO BYPASS	Area Impervious (ha)	None	None	0.029
Urban	LANDSCAPING TO BYPASS	Area Pervious (ha)	None	None	0.098
Urban	LANDSCAPING TO BYPASS	Total Area (ha)	None	None	0.128
Urban	LANDSCAPING TO INFILTRATION TRENCH	Area Impervious (ha)	None	None	0.145
Urban	LANDSCAPING TO INFILTRATION TRENCH	Area Pervious (ha)	None	None	0.016
Urban	LANDSCAPING TO INFILTRATION TRENCH	Total Area (ha)	None	None	0.162
Urban	LANDSCAPING TO OSD	Area Impervious (ha)	None	None	0.028
Urban	LANDSCAPING TO OSD	Area Pervious (ha)	None	None	0.081
Urban	LANDSCAPING TO OSD	Total Area (ha)	None	None	0.11
Urban	NEW ADDITIONS TO EXISTING UNITS	Area Impervious (ha)	None	None	0.026
Urban	NEW ADDITIONS TO EXISTING UNITS	Area Pervious (ha)	None	None	0.004
Urban	NEW ADDITIONS TO EXISTING UNITS	Total Area (ha)	None	None	0.031
Urban	NEW CARPARK	Area Impervious (ha)	None	None	0.003
Urban	NEW CARPARK	Area Impervious (ha)	None	None	0.003
Urban	NEW CARPARK	Area Pervious (ha)	None	None	0
Urban	NEW CARPARK	Total Area (ha)	None	None	0.003
Urban	NEW CARPARK	Total Area (ha)	None	None	0.003
Urban	ROAD TO BYPASS	Area Impervious (ha)	None	None	0.014

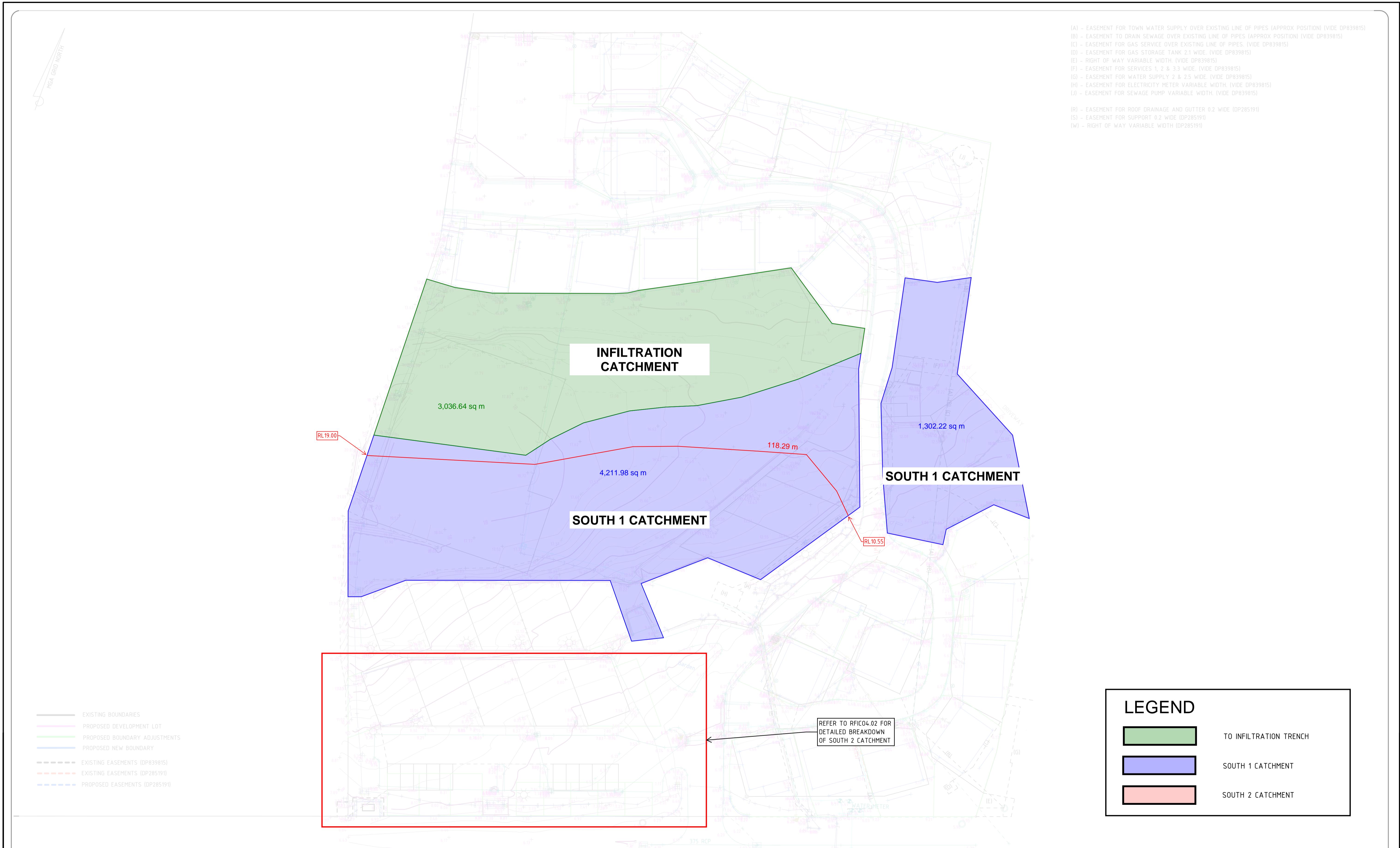
Only certain parameters are reported when they pass validation

Node Type	Node Name	Parameter	Min	Max	Actual
Urban	ROAD TO BYPASS	Area Pervious (ha)	None	None	0
Urban	ROAD TO BYPASS	Total Area (ha)	None	None	0.014
Urban	ROAD TO INFILTRATION	Area Impervious (ha)	None	None	0.078
Urban	ROAD TO INFILTRATION	Area Pervious (ha)	None	None	0
Urban	ROAD TO INFILTRATION	Total Area (ha)	None	None	0.078
Urban	ROAD TO OSD	Area Pervious (ha)	None	None	0.018
Urban	ROAD TO OSD	Total Area (ha)	None	None	0
Urban	ROAD [OSD 2]	Area Impervious (ha)	None	None	0.018
Urban	ROAD [OSD 2]	Area Pervious (ha)	None	None	0.021
Urban	ROAD [OSD 2]	Total Area (ha)	None	None	0
Urban	ROAD [OSD 3]	Area Impervious (ha)	None	None	0.021
Urban	ROAD [OSD 3]	Area Pervious (ha)	None	None	0.01
Urban	ROAD [OSD 3]	Total Area (ha)	None	None	0.01
Urban	ROAD [OSD 4]	Area Impervious (ha)	None	None	0
Urban	ROAD [OSD 4]	Area Pervious (ha)	None	None	0
Urban	ROAD [OSD 4]	Total Area (ha)	None	None	0.01
Urban	ROAD TO BYPASS	Area Impervious (ha)	None	None	0.057
Urban	ROAD TO BYPASS	Area Pervious (ha)	None	None	0
Urban	ROAD TO INFILTRATION TRENCH	Total Area (ha)	None	None	0.057
Urban	ROAD TO INFILTRATION TRENCH	Area Impervious (ha)	None	None	0
Urban	ROAD TO INFILTRATION TRENCH	Area Pervious (ha)	None	None	0.057
Urban	ROAD TO OSD TANK	Total Area (ha)	None	None	0.071
Urban	ROAD TO OSD TANK	Area Impervious (ha)	None	None	0
Urban	ROAD TO OSD TANK	Area Pervious (ha)	None	None	0.071
Urban	ROAD TO RWI / INFILTRATION TRENCH	Area Impervious (ha)	None	None	0.238
Urban	ROAD TO RWI / INFILTRATION TRENCH	Area Pervious (ha)	None	None	0
Urban	ROAD TO RWI / INFILTRATION TRENCH	Total Area (ha)	None	None	0.238

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Failing Parameters					
Node Type	Node Name	Parameter	Min	Max	Actual
Sedimentation	SF Chamber 3m♦ [OSD 1]	Notional Detention Time (hrs)	8	12	0.043
Sedimentation	SF Chamber 3m♦ [OSD 1]	Total Nitrogen - k (m/yr)	500	500	1
Sedimentation	SF Chamber 3m♦ [OSD 1]	Total Phosphorus - k (m/yr)	6000	6000	1
Sedimentation	SF Chamber 3m♦ [OSD 1]	Total Suspended Solids - k (m/yr)	8000	8000	1
Sedimentation	SF Chamber 5m♦	Notional Detention Time (hrs)	8	12	0.0432
Sedimentation	SF Chamber 5m♦	Total Nitrogen - k (m/yr)	500	500	1
Sedimentation	SF Chamber 5m♦	Total Phosphorus - k (m/yr)	6000	6000	1
Sedimentation	SF Chamber 5m♦	Total Suspended Solids - k (m/yr)	8000	8000	1
Sedimentation	SF PRECAST 1200x1200 PRT	Notional Detention Time (hrs)	8	12	0.0757
Sedimentation	SF PRECAST 1200x1200 PRT	Total Nitrogen - k (m/yr)	500	500	1
Sedimentation	SF PRECAST 1200x1200 PRT	Total Phosphorus - k (m/yr)	6000	6000	1
Sedimentation	SF PRECAST 1200x1200 PRT	Total Suspended Solids - k (m/yr)	8000	8000	1

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1	ISSUED FOR INFORMATION	JK	TVK		07.04.26

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PTY LTD

ARCHITECT
EJE architecture

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Email newcastle@northrop.com.au ABN 81 094 433 100

PROJECT
WANDA BEACH ESTATE
FLEET STREET, SALAMANDER BAY
NSW 2317

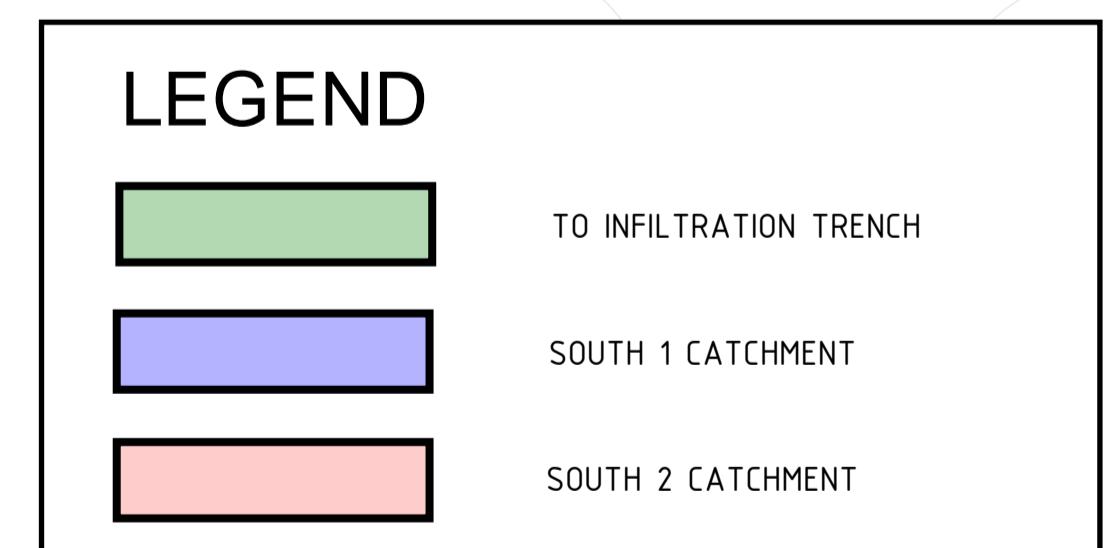
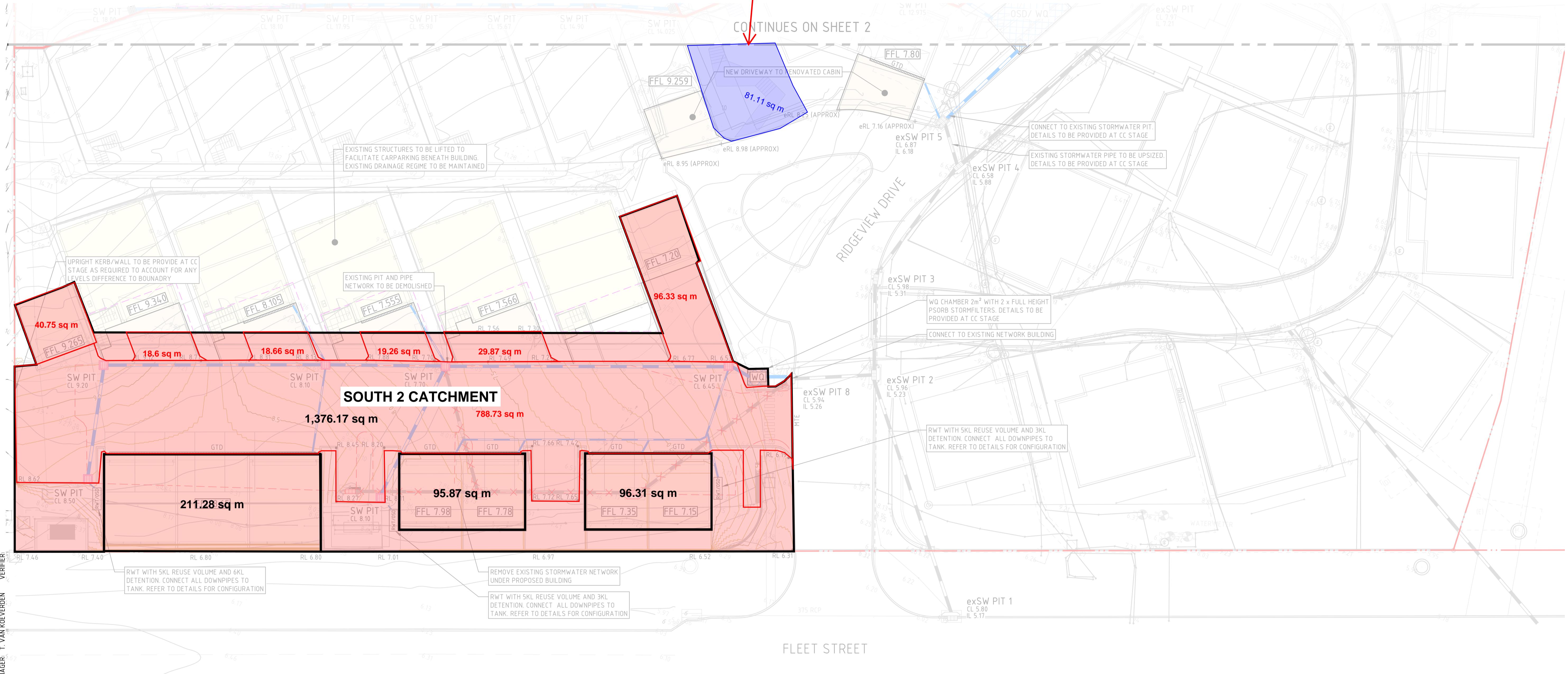
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DRAINS PREDEVEOPED
CATCHMENT PLAN

JOB NUMBER
NL229999

DRAWING NUMBER
RFIC04.01

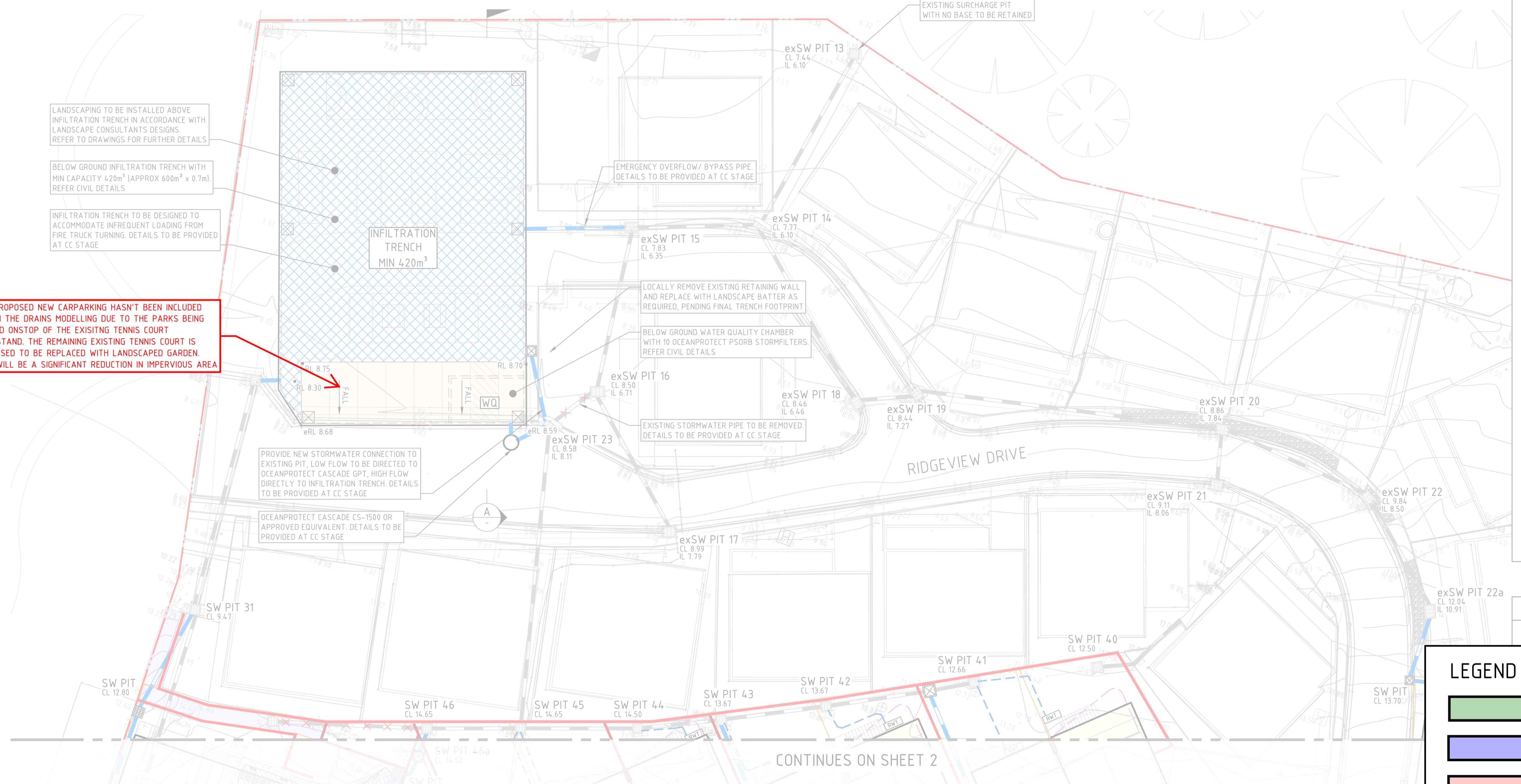
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REVISION	DESCRIPTION	ISSUED	VER'D	APP'D	DATE	CLIENT	ARCHITECT	NOTES	PROJECT	DRAWING TITLE	JOB NUMBER	
1	ISSUED FOR INFORMATION	JK	TVK		07.04.25	WANDA BEACH ESTATE PTY LTD	EJE architecture	ALL DIMENSIONS TO BE VERIFIED ON SITE BEFORE NORTHROP ACCEPTS NO RESPONSIBILITY FOR THE USABILITY, COMPLETENESS OR SCALE OF DRAWINGS TRANSFERRED ELECTRONICALLY. THIS DRAWING MAY HAVE BEEN PREPARED USING COLOUR, AND MAY BE INCOMPLETE IF COPIED TO BLACK & WHITE.	NORTHROP Newcastle Level 1, 215 Pacific Hwy, Charlestown NSW 2290 Ph (02) 4943 1777 Email newcastle@northrop.com.au ABN 81 094 433 100	WANDA BEACH ESTATE FLEET STREET, SALAMANDER BAY NSW 2317	CIVIL ENGINEERING PACKAGE DRAINS PREDEVEOPED CATCHMENT PLAN	NL221140
						DRAWING NOT TO BE USED FOR CONSTRUCTION UNLESS VERIFICATION SIGNATURE HAS BEEN ADDED	THE COPYRIGHT OF THIS DRAWING REMAINS WITH NORTHROP CONSULTING ENGINEERS PTY LTD	SCALE 1:200 @ A1	0 2 4 6 8 10m	DRAWING NUMBER RFIC04.02	REVISION 1	
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1	ISSUED FOR INFORMATION	JK	

25
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PTY LTD**

EJE architecture

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The logo for Northrop Newcastle. It features a red horizontal bar with a white circle on the left side. The word "NORTHROP" is written in large, white, sans-serif capital letters across the bar. Below the bar, the word "Newcastle" is written in a smaller, white, sans-serif font.

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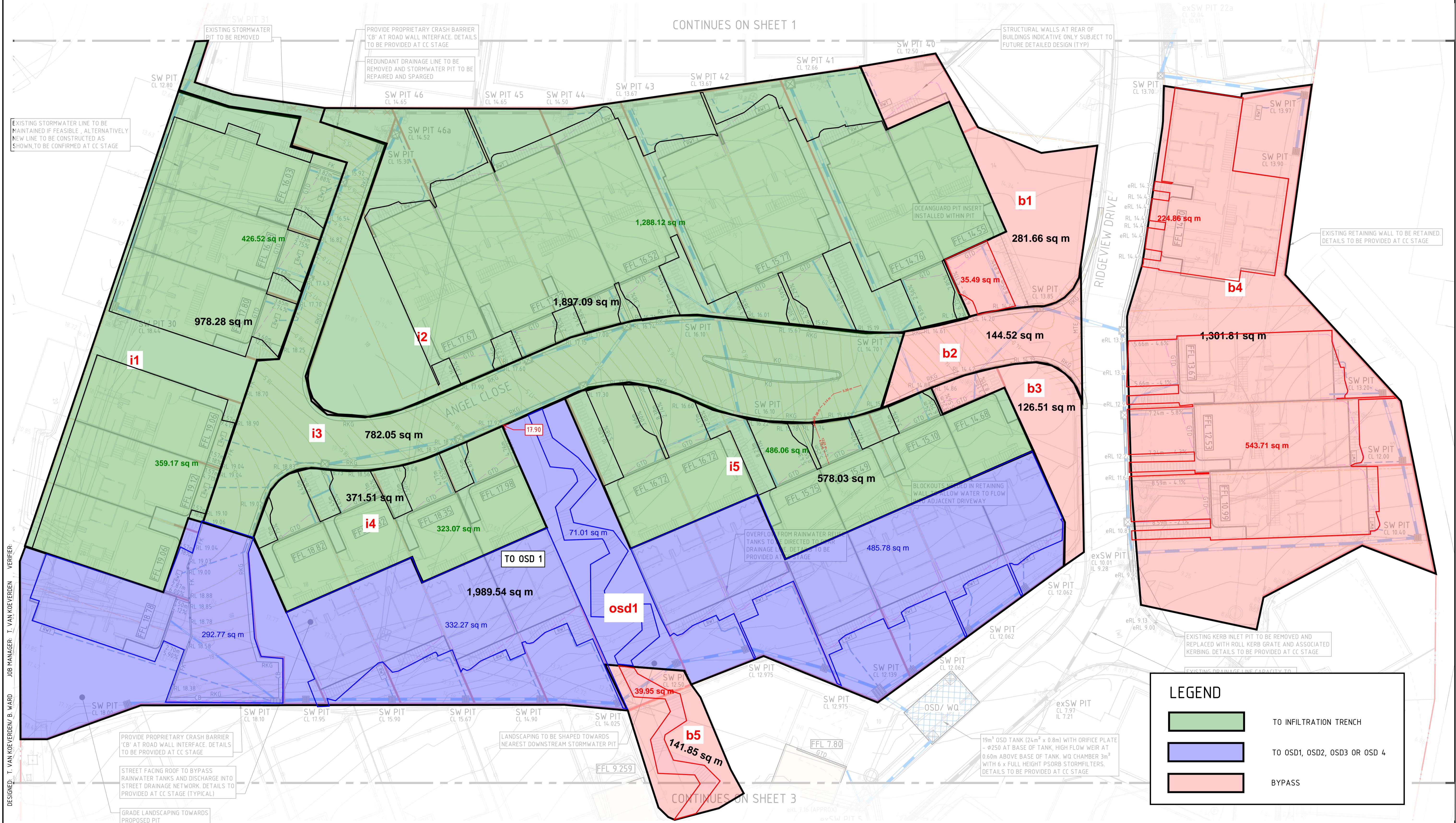
WANDA BEACH ESTATE FLEET STREET, SALAMANDER BAY NSW 2317

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ENGINEERING PACKAGE	JOB NUMBER	NL221140
AINS CATCHMENT PLAN	DRAWING NUMBER	REVISION
RFIC03.01		1
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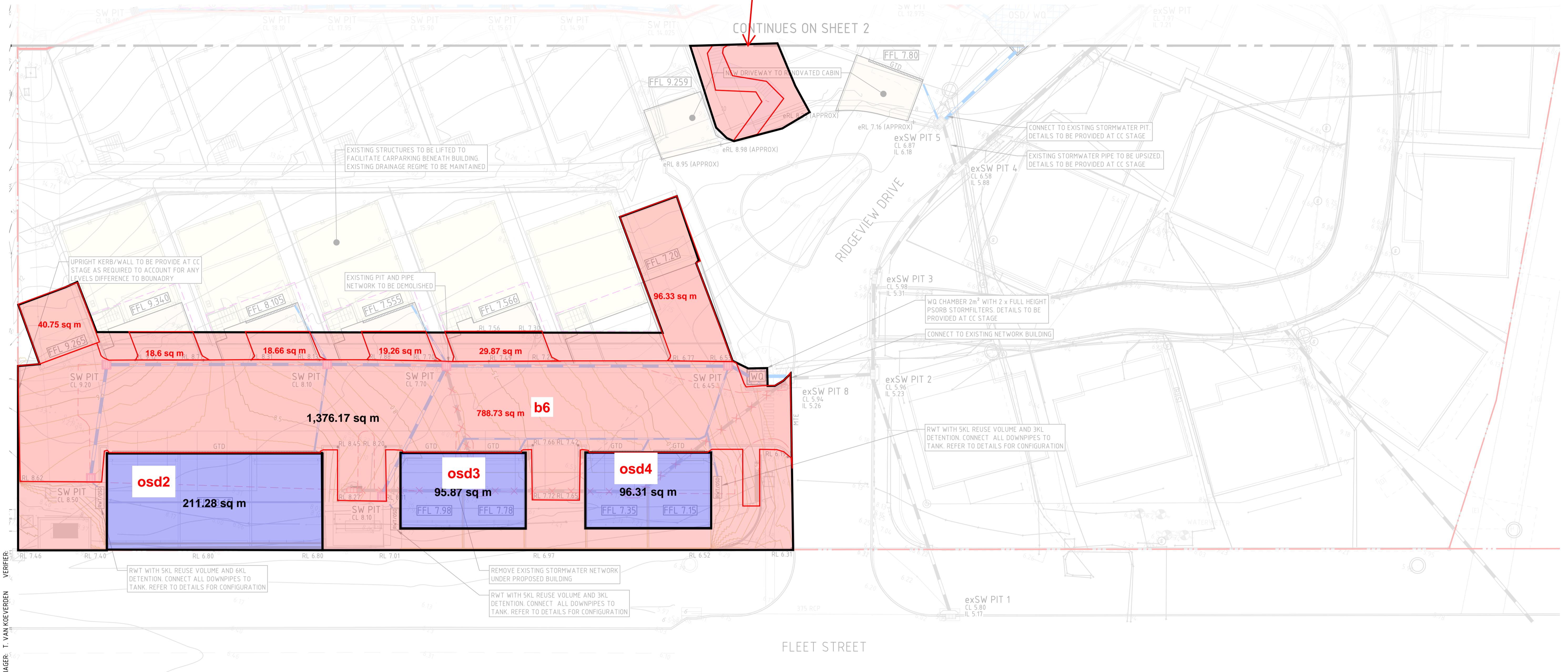
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									DRAWING NUMBER
									REVISION
								RF 103.02	1
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LEGEND

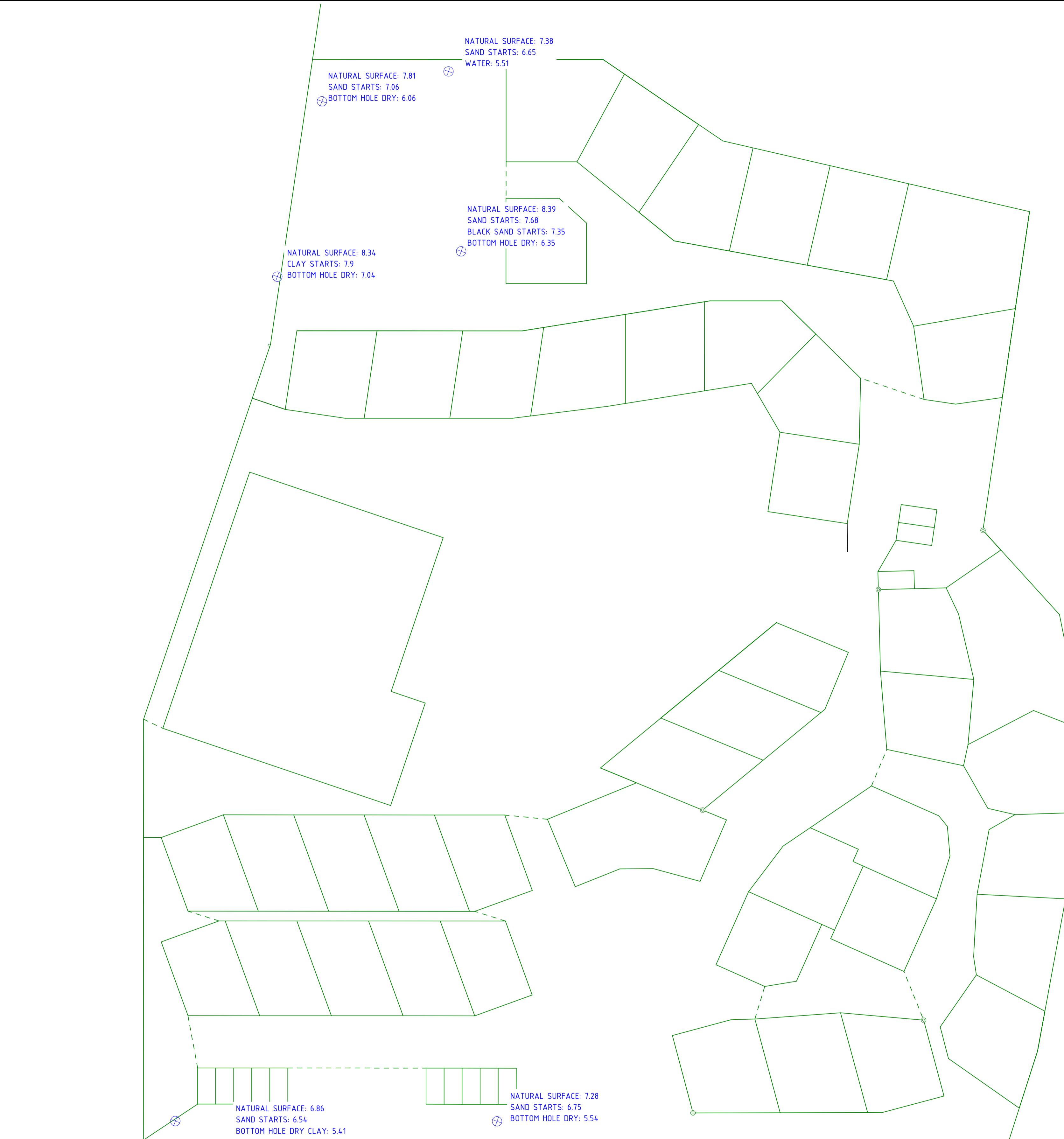
TO INFILTRATION TRENCH

TO OSD1 OSD2 OSD3 OR OSD 4

BYPASS

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1	ISSUED FOR INFORMATION	JK	TVK		07.04.25				DRAWING NUMBER RF103.0	
								REVISION 1		
								DRAWING SHEET SIZE = A1		
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SURVEYORS

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PO Box 418
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0 4 8 12 16 20 40
SCALE 1:400 (A1)
1:800 (A3)
PLAN DATUM AHD
SSM 68070 @ RL 6.016 SCIMS 18/07/2022

SURVEYED BY BM REV AMENDMENTS BY DATE
SURVEYED ON 21/02/2023
DESIGNED -
DRAWN MB
PASSED D CANT

L.G.A. PORT STEPHENS
LOCALITY SALAMANDER BAY
PARISH TOMAREE
COUNTY GLOUCESTER

PROJECT TEST PITS UPON DP258191
LOCATION 4 FLEET ST, SALAMANDER BAY
CLIENT Colonial Ridge Resort

SHEET No. 1 of 1 Revision 0
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